



# Southern Water Integrated Constructed Wetland

**Technical Note: Greenfield Runoff Rates** 

May 2024





#### 1. INTRODUCTION

Greenfield runoff-rates have been calculated for a site proposed for a wetland treatment facility adjacent to Staplefield WTW. An assessment of the impact on greenfield runoff rates post-development has also been made.

#### 2. EXISTING GREENFIELD RUNOFF-RATES

Staplefield Greenfield runoff rates have been calculated using three methods:

- 1) IoH124 report
- 2) FEH method
- 3) ReFH2 rainfall runoff (plot scale equations using FEH22 rainfall)

Methods 1 and 2 have been carried out using the tool provided by the UK SUDS website: <a href="https://www.uksuds.com/">https://www.uksuds.com/</a>. The pro forma are included in Appendix A. The greenfield runoff rates are calculated for the entire site area (3.13 ha) and can then be calculated for each component of the site (wetland treatment cell, flood mitigation pond, gravelled areas) on a pro rata by area basis if required (refer to Appendix B). The site layout is provided in Dwg. no. 23539\_4\_13 SURFACE WATER MANAGEMENT; refer to Appendix C. The ReFH2 method uses plot scale calculations using catchment descriptors from a small nearby catchment imported into the ReFH2.3 software and then similarly calculated on a pro rata by area basis. The greenfield runoff rates for the whole site are presented in Table 1 below for different events and are attributable to the existing site, which is an agricultural field. The QBAR event is included, which is the mean annual maximum flow rate.

Table 1: Greenfield runoff rates

Return Period	IoH124	FEH	ReFH2
[years]	l/s	l/s	l/s
1	15.9	21.1	18.5
QBAR	18.7	24.9	21.0
30	43.1	57.2	44.4
100	59.8	79.3	56.2

#### 3. POST-DEVELOPMENT RUNOFF-RATES

The agricultural field drains southwards into the River Ouse. The creation of a treatment wetland will effectively create attenuation ponds into which the majority of the rainfall across the site will end up, either directly or through runoff. The treatment wetland has a minimum travel time of 2 days and is designed not to exceed the current discharge permit of 4 l/s, which is substantially lower than the existing greenfield runoff rates shown in Table 1 above. The flood mitigation area will provide over 4000 m³ of storage, which will also slow the rate of flow into the Ouse. Around 20% of the site will continue to drain without attenuation into the River Ouse, the majority of which will be grassed, with a small proportion gravelled. Gravelled areas will be permeable, which will continue to allow infiltration, so its impact on runoff rates will be small. The runoff rates to the River Ouse will therefore not increase in comparison to existing greenfield runoff rates once the treatment wetland is constructed.



## **APPENDIX A.1: UK SuDS IoH124**

rwalling	ford				nfield runoff imation for s				
ıı wanıng	ioiu			www.uksuds.	.com   Greenfie <b>l</b> d run				
alculated by:	Timoth	y Paddison		Site Details					
te name:	Staplef	ord		Latitude:	51.03170° N				
te location:	Cuckfie	ld Road		Longitude:	0.17567° W				
iteria in line with Er evelopments", SC03 andards for SuDS (	nvironment 30219 (2013 Defra, 2015	: Agency guidance "R ) , the SuDS Manua <b>l</b> C	ainfal <b>l</b> runoff man 753 (Ciria, 2015) aı n greenfie <b>l</b> d runof	nd the non-statutory if rates may be the basis Date:	808248419 Dec 01 2023 15:29				
unoff esti	matior	n approach	IH124						
ite charac	teristi	cs		Notes					
otal site area (h	a): 3.134	15		(1) Is Q <sub>BAR</sub> < 2.0 I/s/ha?					
1ethodo <b>l</b> og	gy								
<sub>BAR</sub> estimation m	R estimation method: Calculate from SPR and SAAR			When Q <sub>BAR</sub> is < 2.0 I/s/ha then limiting discharge rates are set at 2.0 I/s/ha.					
PR estimation m	ethod:	Calculate from	SO <b>I</b> L type	Tates are set at 2.0 f/s/fla.					
oi <b>l</b> charac	teristi	CS Defau <b>l</b> t	Edited	(2) Are flow rates < 5.0 <b>l</b> /s	5?				
OIL type:		4	4	When flourete as less than 5.01/2 and at					
OST class:		N/A	N/A	Where flow rates are less than 5.0 l/s consent for discharge is usually set at 5.0 l/s if blockage					
PR/SPRHOST:		0.47	0.47	from vegetation and other materials is possible.  Lower consent flow rates may be set where the					
lydrologica				blockage risk is addressed by us					
haracteris	STICS	Default	Edited	drainage elements.					
AAR (mm):		840	840						
ydrological regio	on:	7	7	(3) Is SPR/SPRHOST ≤ 0.3?					
rowth curve fac	tor 1 year	0.85	0.85	Where groundwater levels are lo	w enough the				
rowth curve fac ears:	tor 30	2.3	2.3	use of soakaways to avoid discharge offsite					
rowth curve fac ears:	tor 100	3.19	3.19	would normally be preferred for disposal of surface water runoff.					
	tor 200	3.74	3.74						



Q <sub>BAR</sub> (I/s):	18.73	18.73
1 in 1 year (I/s):	15.92	15.92
1 in 30 years (I/s):	43.08	43.08
1 in 100 year (I/s):	59.75	59.75
1 in 200 years (l/s):	70.05	70.05

This report was produced using the greenfield runoff tool developed by HR Wallingford and available at www.uksuds.com. The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at www.uksuds.com/terms-and-conditions.htm. The outputs from this tool are estimates of greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, CEH, Hydrosolutions or any other organisation for the use of this data in the design or operational characteristics of any drainage scheme.



## **APPENDIX A.2: UK SuDS FEH**

hrwalling	ford			•	estimation for s		
	•			www.uk	suds.com   Greenfie <b>l</b> d rund		
Calculated by:	Timothy	Paddison		Site Det	ails		
Site name:	Staplefo	ord		Latitude:	51.03170° N		
Site location:	Cuckfiel	d Road		Longitude:	0.17567° W		
his is an estimation	n of the gree	nfie <b>l</b> d runoff ra	ates that are used to note "Rainfa <b>ll</b> runoff man	neet normal best practice Reference:	2485487649		
leve <b>l</b> opments", SC0 tandards for SuDS	(2013) (2013) (Defra, 2015)	, the SuDS Man This information	ua <b>l</b> C753 (Ciria, 2015) aı	nd the non-statutory  If rates may be the basis Date:	Dec 01 2023 16:04		
Runoff esti approach	imation		FEH Statistical				
Site charac	cteristic	cs		Notes			
otal site area (ha): 3.1345				(1) Is Q <sub>BAR</sub> < 2.0 I/s/ha?			
Methodo <b>l</b> ogy							
MED estimation method: Calculate from BFI and SA		om BFI and SAAR	When Q <sub>BAR</sub> is < 2.0 I/s/ha then limiting discharge rates are set at 2.0 I/s/ha.				
Fland SPR method: Specify BFI manually		manua <b>li</b> y	14.00 die 301 dt 2.0 g 3/14.				
IOST class:	OST class:		(2) Are flow retoo . F.01/22				
FI / BFIHOST:		0.337		(2) Are flow rates < 5.0 <b>l</b> /s?			
MED (I/s):		16.77		Where flow rates are less than 5.0 l/s consent for discharge is usually set at 5.0 l/s if blockage from vegetation and other materials is possible.			
O <sub>BAR</sub> / Q <sub>MED</sub> facto	or:	1.14					
Hydrologica characteris		Defau	lt Edited	Lower consent flow rates r blockage risk is addressed	3 C C C - C C C C C C C C C C C C C C C		
SAAR (mm):		840	840	drainage elements.			
łydrological regi	ion:	7	7	(3) Is SPR/SPRHOST ≤ (	າ 32		
Growth curve fac	ctor 1 year.	0.85	0.85	(0) 13 01 11/07 1111001 = (	5.0:		
Growth curve factor 30 2.3 ears:		2.3	Where groundwater levels are low enough the use of soakaways to avoid discharge offsite				
Growth curve fac ears:	ctor 100	3.19	3.19	would normally be preferre surface water runoff.	d for disposa <b>l</b> of		
Frowth curve factor 200 3.74 3.74		3.74	7				



Q <sub>BAR</sub> (I/s):	19.06	24.85
1 in 1 year (I/s):	16.2	21.12
1 in 30 years (I/s):	43.83	57.16
1 in 100 year (I/s):	60.79	79.28
1 in 200 years (I/s):	71.27	92.94

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## **Appendix B: Staplefield WTW Components**

#### **SUDS Tool – IoH124**

Area (ha) =		0.0482	0.5399	0.4418	0.2585	1.3108	0.2555	0.1852	0.093	
Event	Site Area		Cell 1	Cell 2	Cell 3	Cell 4	Grassed	Gravel to treatment	Gravel to grass	Flood Mitigation
(1 in x years)	l/s	l/s/ha	l/s	l/s	l/s	l/s	l/s	l/s	l/s	l/s
1	15.92	5.1	0.24	2.74	2.24	1.31	6.66	1.30	0.94	0.47
QBAR	18.73	6.0	0.29	3.23	2.64	1.54	7.83	1.53	1.11	0.56
30	43.08	13.7	0.66	7.42	6.07	3.55	18.02	3.51	2.55	1.28
100	59.75	19.1	0.92	10.29	8.42	4.93	24.99	4.87	3.53	1.77
200	70.05	22.3	1.08	12.07	9.87	5.78	29.29	5.71	4.14	2.08

#### **SUDS Tool – FEH method**

Area (ha) =		0.0482	0.5399	0.4418	0.2585	1.3108	0.2555	0.1852	0.093	
Event	Site	Site Area		Site Area Cell 1 Cell 2 Cell 3 Cell 4	Grassed	Gravel to treatment	Gravel to grass	Flood Mitigation		
(1 in x years)	l/s	l/s/ha	l/s	l/s	l/s	l/s	l/s	l/s	l/s	l/s
1	21.12	6.7	0.32	3.64	2.98	1.74	8.83	1.72	1.25	0.63
QBAR	24.85	7.9	0.38	4.28	3.50	2.05	10.39	2.03	1.47	0.74
30	57.16	18.2	0.88	9.85	8.06	4.71	23.90	4.66	3.38	1.70
100	79.28	25.3	1.22	13.66	11.17	6.54	33.15	6.46	4.68	2.35
200	92.94	29.7	1.43	16.01	13.10	7.66	38.87	7.58	5.49	2.76



## **ReFH2 – plot scale equations**

		Area (ha) =	0.0482	0.5399	0.4418	0.2585	1.3108	0.2555	0.1852	0.093
Event	Site Area		Cell 1 Cell 2	Cell 2	Cell 3	Cell 4	Grassed	Gravel to	Gravel to	Flood
(1 in x years)	l/s	l/s/ha	l/s	l/s	l/s	l/s	l/s	treatment I/s	grass I/s	Mitigation I/s
1	530	5.9	0.28	3.19	2.61	1.53	7.74	1.51	1.09	0.55
QBAR	601	6.7	0.32	3.62	2.96	1.73	8.78	1.71	1.24	0.62
30	1270	14.2	0.68	7.64	6.25	3.66	18.55	3.62	2.62	1.32
100	1610	17.9	0.86	9.69	7.93	4.64	23.51	4.58	3.32	1.67
200	1840	20.5	0.99	11.07	9.06	5.30	26.87	5.24	3.80	1.91

<sup>\*</sup>Runoff rates calculated from nearest small catchments, which has a site area of 89.75ha.



# **Appendix C: Site Layout**

